Seat No.:			-	Enrolment No				
		GUJ	ARAT TECHNOI	LOGI	CAL UNIVERSITY			
]	BE - SI	EMESTER-V (NEW) - 1	EXAMI	NATION – SUMMER 2017			
Subject			` ,		Date: 10/05/201	7		
•					Date: 10/03/201	. ,		
U			Soil Mechanics					
Time: 0	2:3	0 PM	to 05:00 PM		Total Marks: 7	70		
Instructio	ns:							
1.	At	tempt	all questions.					
			table assumptions wherev		ary.			
3.	Fi	gures t	o the right indicate full ma	rks.				
Q.1		Answ	ver the following			1-		
٧.,	1		unit weight of soil at zero	air voic	ls depends on			
	-	I A	Specific gravity	C	Water content			
		В	Unit weight of water	D _	AB of these			
	2	Com	naction by vibratory rol	ler is the	best method of compaction in c	use		
	-	of						
		Í.A.	Moist silty sand	[C	Clay of medium compressibilit	y .		
		В	Well graded dry sand	· D	Clay of medium compressibility	i		
	3	The	vertical stress at depth	z direc	tly below the point load P is (I	i is		
		const	tant)	1	1			
		A	k*(P/z)	. C	k*(P/x²) k*(P/xz)			
	4	B A 25	k*(P/z³) kN point load act on th	⊢ U e surfac	e of an infinite clastic medium.	The		
	•	at a point 6.0 m below and 4.0	m					
			from the load will be					
			132	. (13.2			
		B	1.32	† D	0.1.32			
	5	A 41	seet cheen tost was cond	lucted o	n a cohesionless soil (c=0) specie	nen		
	5 A direct shear test was conducted on a cohesionless soil (c=0) under a normal stress of 200 kN/m², the specimen failed at a sh							
			00 kN/m ³ , the angle of int					
		A	26.6°	C	30.0°			
		В	29.5°	D	32.6°			
				-1	the faith fraction and a the fail	in re-		
-	- 6	For	a sample of dry conesion	micss so maior pr	il with friction angle, Φ, the fail incipal plane by an angle equal to	0		
		A	Φ		45°			
		В	45°- Ф/2	Ď	45° + Φ/2			
	_							
	7	1 -	-	tor dete	rmining in situ shear strength of Stiff clay			
		A	Soft clay sand	⊢ E	Gravel			
		; B			1			
	8	The	The earth pressure for the design of bridge abutments is taken as					
		Α.	Active thrust	€.	Thrust in at rest condition			
		В	Passive thrust	D	None of the above			
	9 Coulomb's theory of earth pressure is based on							
	-	[A	The theory of elastici		Empirical rules			
		В	The theory of plastici		Wedge theory			

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,	10	A deep cut of 7 m has to be made in a clay with unit weight of 16 kN/m ³						
		and a cohesion of 25 kN/m ² . What will be the factor of safety is one has to						
		have a slope angle 30°? Stability number is given to be 0.178(from						
		Taylor's chart) for a depth factor of 3.						
		A 0.8 C 1.25 D 1.0						
		B 11.1						
	11	In friction circle method of slope analysis, if r defines the radius of the						
	• •	slip circle, the radius of friction circle is						
		A r sinΦ C r cosΦ						
		B r D r tanΦ						
		A footing 2m x 1m exerts a uniform pressure of 150 kN/m² on the soil.						
	12	Assuming a load dispersion of 2 vertical to 1 horizontal, the average						
		vertical stress (kN/m²) at 1.0 m below the footing is						
		·						
		A 50 B 75 D 100						
		15 16						
	1.3	The measure of soil compaction is its wet density. (True/Fulse)						
	14	A foundation is considered as shallow if its depth is						
		A less than I meter C equal to or less than its						
		width						
		B prenter than its width D prenter than I meter	2					
Q.2	(a)	In a diandard proclut lest 1.0 kg of moist son was though in master						
		(volume=944 ec) after compaction. A soil sample weighting 23 g was						
		taken from the mould and ovendried for 24 hours at a temperature of						
		110°C. Weight if the dry sample was found to be 20 g, specific gravity of soil solids is G=2.7. Find out theoretical maximum value of the dry unit						
		soil solids is G=2.7, Find out incorrection maximum value of the off						
		weight of the soil at that water content. Enlist and explain factors affecting compaction.	4					
	(b)	the second second second second second by the second secon	7					
	(c)	vertical stress increment at points directly beneath the load upto a depth						
		of 10m with an interval of 1 m and draw a plot.						
		OR						
	6.03	Plot the variation of vertical stress increment due to lond on horizontal (17					
	(0)	planes at depths of 1 m, 2 m and 3 m upto μ horizontal distance of 3						
		for 1 m interval on either side of centre.						
Q.3	(w)	Write limitations of shear nox tests.	1,3					
4	(b)	Enlist and explain shear tests bases on drainage condition.)4					
	(c)	45	17					
	1-7	•						
		OR						
			11.3					
Q.3	(a)	the a telestal sport lest compacted on a same absorber of a second						
		pressure of 100 kN/m2 under drained condition, resulted in deviator						
		stress (\sigma_1-\sigma_3) at failure of 100 kN/m', Find the angle of shearing						
		resistance.	04					
	(b)	Following results were obtained from an obstracted sites.						
		Soil Novembland (N) 250 500 750						
		Normal mad (14)						
		Determine the strength parameters in terms of total stresses. The c/s area						
		of shear bux was 36 cm ² .						

7/-		1, (41)		
Q.4	(c) (a)	Explain direct shear box test. An unsupported excavation is made to the maximum possible depth in a clay soil having γ_t =18 kN/m ³ , c = 100 kN/m ² , Φ =30°. Find out active earth pressure, according to Rankine's theory, at the base level of the exaction.	07 03	
	(b) (c)	Write about Initial, primary and secondary consolidation. Derive equation for total force due to earth pressure at rest.	07	
		OR		
Q.4	(a)	A retaining wall of height 8 m retains dry sand. In the initial state, the soil is loose and has a void ratio of 0.5, $\gamma_d = 17.8 \text{kN/m}^3$ and $\Phi = 30^\circ$. Subsequently, the backfill is compacted to a state where void ratio is 0.4, $\gamma_d = 18.8 \text{kN/m}^3$ and $\Phi = 35^\circ$. Find out the ratio of passive thrust to the final passive thrust, according to Rankine's earth pressure theory.	03	
	(b) (c)	Determine the active pressure on the retaining wall shown in Fig. 1. A clay layer whose total settlement under a given loading is expected to be 12 cm settles 3 cm at the end of 1 month after the application of load increment. How many months will be required to reach a settlement of 6 cm? How much settlement will occur in 10 months? Assume the layer to have double drainage. (For T ₁ = 0.492 1)=76%)	07	
Q.5	(a) (b)	clay 3 m thick. The sample was drained at top and bottom. The transfer of the sample was observed to be 15	03 04	
	(e)	minutes. Determine the coefficient of consolidation of clay.	07	
		OR		
Q.5	5 (a) (b)	A long natural slope in an overconsolidated chay ($\epsilon = 10^{\circ} \text{ keV}$) is inclined at 10° to the horizontal. The water table is at the surface and the seepage is parallel to slope. If a plane slip had developed at a depth of 5 m below the surface, determine the factor of		
	(e	safety. Take y _w =10kN/m".	07	
		Φ'=35° γ = 17 kN/m³ 2.5 m Φ'=38° γ = 18 kN/m³ C		

FIG-1